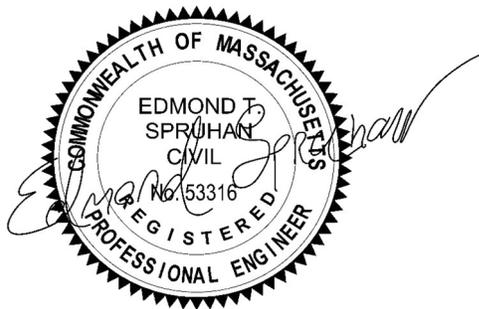


SPRUHAN ENGINEERING, P.C.

# PIPE FLOW CALCULATIONS

500 SCHOOL ST, MANSFIELD, MA



**Prepared By: Spruhan Engineering**

Date: July 28th, 2020. ; Revised: September 1st, 2020.

To whom it may concern:

The following information prepared by **Spruhan Engineering, P.C.** presents the pipe flow calculations for the proposed development located at 500 School St, Mansfield, Massachusetts.

**Roof:**

The pipe under evaluation is a 12" PVC Blue Brute drainage pipe, which has a Manning's roughness coefficient of 0.009, and it must be capable of transferring water collected from the building roof with a drainage area of 44,266 S.F. and a Flow Rate of 5.75 cfs to a subsurface infiltration system.

**Total Required Flow Rate** = 5.75 Cfs / 2 pipes = 2.87 cfs

**Pipe provided Volume** = 4.69 cfs (See Appendix A for Calculations)

**Driveway:**

The pipe under evaluation is a 12" RCP drainage pipe, which has a Manning's roughness coefficient of 0.012, and it must be capable of transferring water collected from 2 driveways with a drainage area of 10,814 S.F. and a Flow Rate of 1.41Cfs to a subsurface infiltration system.

**Total Required Flow Rate** = 1.41 cfs / 2 pipes = .7 cfs

**Pipe provided Volume** = 2.48 cfs (See Appendix A for Calculations)

**Parking:**

The pipe under evaluation is a 12" RCP drainage pipe, which has a Manning's roughness coefficient of 0.012, and it must be capable of transferring water collected from a paved parking with a drainage area of 16,774 S.F. and a Flow rate of 2.18 cfs to a subsurface infiltration system.

**Total Required Flow Rate** = 2.18 cfs

**Pipe provided Volume** = 2.48 cfs (See Appendix A for Calculations)

HydroCAD was used to calculate the peak discharge and a spreadsheet that uses Manning's equation to calculate pipe flow capacity. These calculations were carried out using a rainfall intensity of 5.6 inches/hour for a 25 Year Storm Event to show that the proposed pipe can cope with peak discharge from the two driveways at 75% capacity and the minimum Slope to be more conservative, therefore Spruhan Engineering, P.C. recommend the use of this pipe, further information is shown in Appendix A below.

## APPENDIX A

## Manning Formula Uniform Pipe Flow at Given Slope and Depth

### Inputs:

Pipe Diameter, $d_o$	12.0000	in
Manning Roughness, $n$	0.0120	
Pressure slope (possibly equal to pipe slope), $S_o$	0.5000	% slope
Percent of (or ratio to) full depth (100% or 1 if flowing full)	0.7500	fraction

### Results:

Flow, $Q$	2.4886	ft <sup>3</sup> /s
Velocity, $v$	1.2005	m/s
Velocity head, $h_v$	0.0735	m
Flow Area, $A$	0.0587	m <sup>2</sup>
Wetted Perimeter, $P$	0.6384	m
Hydraulic Radius	0.0920	m
Top Width, $T$	0.2640	m
Froude Number, $F$	0.81	
Shear Stress (tractive force), $\tau$	11.2083	N/m <sup>2</sup>

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HawsEDC Calculators

## Manning Formula Uniform Pipe Flow at Given Slope and Depth

### Inputs:

Pipe Diameter, $d_o$	12.0000	in
Manning Roughness, $n$	0.0090	
Pressure slope (possibly equal to pipe slope), $S_o$	1.0000	% slope
Percent of (or ratio to) full depth (100% or 1 if flowing full)	0.7500	fraction

### Results:

Flow, $Q$	4.6925	ft <sup>3</sup> /s
Velocity, $v$	2.2636	m/s
Velocity head, $h_v$	0.2613	m
Flow Area, $A$	0.0587	m <sup>2</sup>
Wetted Perimeter, $P$	0.6384	m
Hydraulic Radius	0.0920	m
Top Width, $T$	0.2640	m
Froude Number, $F$	1.53	
Shear Stress (tractive force), $\tau$	22.4165	N/m <sup>2</sup>

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